

Plane Stress Assignment 3.1

Plane Strain

$$\frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix}$$

$$\frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & 0 \\ \nu & 1-\nu & 0 \\ 0 & 0 & \frac{1-2\nu}{2} \end{bmatrix}$$

Plane strain can be written as:

$$\frac{E^*}{1-\nu^{*2}} \begin{bmatrix} 1 & \nu^* & 0 \\ \nu^* & 1 & 0 \\ 0 & 0 & \frac{1-\nu^*}{2} \end{bmatrix}$$

$$\frac{E^*}{1-\nu^{*2}} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)} \quad (1)$$

$$\frac{E^*(1-\nu^*)}{2(1-\nu^{*2})} = \frac{E(1-2\nu)}{(1+\nu)(1-2\nu)2} \quad (2)$$

from (1) find  $E^*$

$$\frac{E^*}{(1-\nu^*)(1+\nu^*)} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

$$E^* = \frac{E(1-\nu)(1-\nu^*)(1+\nu^*)}{(1+\nu)(1-2\nu)} \quad (3)$$

Substitute (3) in (2)

$$\frac{E(1-\nu)(1-\nu^*)(1+\nu^*)(1-\nu^*)}{2(1-\nu^{*2})(1+\nu)(1-2\nu)} = \frac{E}{(1+\nu)2}$$

$$\frac{(1-\nu)(1-\nu^*)}{(1-2\nu)} = 1$$

$$(1-\nu)(1-\nu^*) = 1-2\nu$$

$$1-\nu^* = \frac{1-2\nu}{1-\nu}$$

$$\nu^* = -\frac{1-2\nu}{1-\nu} + 1$$

$$= \frac{2\nu-1}{1-\nu} + \frac{1-\nu}{1-\nu}$$

$$= \frac{-1+2\nu+1-\nu}{1-\nu}$$

$$= \frac{(2\nu-\nu)+(1-1)}{1-\nu}$$

$$\boxed{\nu^* = \frac{\nu}{1-\nu}} \quad (4)$$

Substitute (4) in (1)

$$\frac{E^*}{1-\nu^{*2}} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

$$\frac{E^*}{1-\frac{\nu^2}{(1-\nu)^2}} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

$$\frac{E^*(1-\nu)^2}{(1-\nu)^2 - \nu^2} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

$$\frac{E^*(1-\nu)^2}{1-2\nu + \nu^2 - \nu^2} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

$$\boxed{E^* = \frac{E}{(1-\nu)(1+\nu)} = \frac{E}{1-\nu^2}}$$

### Assignment 3.1

Plane Stress can be written as:

$$\frac{E^*}{(1+\nu^*)(1-2\nu^*)} \begin{bmatrix} 1-\nu^* & \nu^* & 0 \\ \nu^* & 1-\nu^* & 0 \\ 0 & 0 & \frac{1-2\nu^*}{2} \end{bmatrix}$$

$$\frac{E^* (1-\nu^*)}{(1+\nu^*)(1-2\nu^*)} = \frac{E}{1-\nu^2} \dots \dots \textcircled{1}$$

$$\frac{E^* (1-2\nu^*)}{2(1+\nu^*)(1-2\nu^*)} = \frac{E(1-\nu)}{(1-\nu^2)2} \dots \dots \textcircled{2}$$

From (1) find  $E^*$

$$E^* = \frac{E(1+\nu^*)(1-2\nu^*)}{(1-\nu^2)(1-\nu^*)} \dots \dots \textcircled{3}$$

Substitute (3) in (2)

$$\frac{E(1+\nu^*)(1-2\nu^*)}{2(1-\nu^2)(1-\nu^*)(1+\nu^*)} = \frac{E(1-\nu)}{(1-\nu^2)2}$$

$$\frac{1-2\nu^*}{1-\nu^*} = 1-\nu$$

$$\frac{\nu^*}{1-\nu^*} = \nu$$

$$\frac{1}{\frac{1}{\nu^*} - 1} = \nu$$

$$\frac{1}{\nu^*} = \frac{1}{\nu} + 1 = \frac{1+\nu}{\nu}$$

$$\boxed{\nu^* = \frac{\nu}{1+\nu}} \textcircled{4}$$

Substitute (4) in (1)

$$\frac{E^*(1+\nu-\nu)}{1+\nu} = \frac{E}{1-\nu^2}$$

$$\frac{E^*(1+\nu)}{(1+2\nu)(1-\nu)} = \frac{E}{1-\nu^2}$$

$$E^* = \frac{E(1-\nu)(1+2\nu)}{(1+\nu)(1-\nu^2)}$$

$$\boxed{E^* = \frac{E(1+2\nu)}{(1+\nu)^2}}$$

Assignment 3.2

$$\lambda = \frac{E\nu}{(1+\nu)(1-2\nu)} \dots (1) \quad \mu = G = \frac{E}{2(1+\nu)} \dots (2)$$

Obtain E :

$$E = \frac{\lambda(1+\nu)(1-2\nu)}{\nu} \dots (3)$$

Obtain  $\nu$

$$\nu = \frac{E}{2\mu} - 1 \dots (4)$$

Substitute (4) in (3)

$$E = \frac{\lambda \left(1 + \frac{E}{2\mu} - 1\right) \left(1 - 2\left(\frac{E}{2\mu} - 1\right)\right)}{\frac{E}{2\mu} - 1}$$

$$E = \frac{\lambda \left(\frac{E}{2\mu}\right) \left(\frac{-2E + 6\mu}{2\mu}\right)}{\frac{E - 2\mu}{2\mu}}$$

$$E = \frac{\lambda E (-2E + 6\mu)}{2\mu (E - 2\mu)}$$

$$E = \frac{\lambda E (-2E + 3\mu)^2}{2\mu (E - 2\mu)} =$$

$$\mu(E - 2\mu) = \lambda(-E + 3\mu)$$

$$-\lambda E - \mu E = -2\mu^2 - 3\lambda\mu$$

$$E = \frac{(3\lambda + 2\mu)\mu}{\lambda + \mu} \dots (5)$$

Substitute (5) in (4)

$$\nu = \frac{(3\lambda + 2\mu)\mu}{(\lambda + \mu)2\mu} - 1$$

$$\nu = \frac{3\lambda + 2\mu - 2(\lambda + \mu)}{2(\lambda + \mu)}$$

$$\nu = \frac{3\lambda + 2\mu - 2\lambda - 2\mu}{2(\lambda + \mu)}$$

$$\nu = \frac{\lambda}{2(\lambda + \mu)}$$

Assignment 3-3

Linear triangular Element

$$K^e = \int_{\Omega^e} h B^T E B d\Omega$$

Bar Element Stiffness matrix

$$K^e = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ 0 & 1 \end{bmatrix}$$

$$K^e = \frac{h}{4A} \begin{bmatrix} y_{23} & 0 & x_{32} \\ 0 & x_{32} & y_{23} \\ y_{31} & 0 & x_{13} \\ 0 & x_{13} & y_{31} \\ y_{12} & 0 & x_{21} \\ 0 & x_{21} & y_{12} \end{bmatrix} \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{12} & E_{22} & E_{23} \\ E_{13} & E_{23} & E_{33} \end{bmatrix} \begin{bmatrix} y_{23} & 0 & y_{31} & 0 & y_{12} & 0 \\ 0 & x_{32} & 0 & x_{13} & 0 & x_{21} \\ x_{32} & y_{23} & x_{13} & y_{31} & x_{21} & y_{12} \end{bmatrix}$$

$\nu = 0$   $E = E \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \frac{1}{2} \end{bmatrix}$   $h = a = 1$   $\therefore A = \frac{1}{2}$

$$K^e = \frac{1}{4 \cdot \frac{1}{2}} \begin{bmatrix} -1 & 0 & -1 \\ 0 & -1 & -1 \\ 1 & 0 & 0 \\ 0 & 0 & 1 \\ 0 & 0 & -1 \\ 0 & 1 & 0 \end{bmatrix} E \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \frac{1}{2} \end{bmatrix} \begin{bmatrix} -1 & 0 & 1 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 1 \\ -1 & -1 & 0 & 1 & 1 & 0 \end{bmatrix}$$

$$K^e = \frac{E}{4} \begin{bmatrix} 3 & 1 & -2 & -1 & -1 & 0 \\ 1 & 3 & 0 & -1 & -1 & -2 \\ -2 & 0 & 2 & 0 & 0 & 0 \\ -1 & -1 & 0 & 1 & 1 & 0 \\ -1 & -1 & 0 & 1 & 1 & 0 \\ 0 & -2 & 0 & 0 & 0 & 2 \end{bmatrix}$$

Stiffness Matrix for Turner triangle

For the model using bar elements

$$\bar{K} = \frac{EA}{L} \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$

Using the respective angles, one obtains the elemental stiffness as  $(T^e)^T K^e T^e$  that is:

$$K^e = \frac{EA}{L} \begin{bmatrix} c^2 & sc & -c^2 & -sc \\ sc & s^2 & -sc & -s^2 \\ -c^2 & -sc & c^2 & sc \\ -sc & -s^2 & sc & s^2 \end{bmatrix}$$

$$k_1 = EA_1 \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & -1 \\ 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 1 \end{bmatrix} \quad k_2 = EA_2 \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \quad k_3 = \frac{EA_3}{2\sqrt{2}} \begin{bmatrix} 1 & -1 & -1 & 1 \\ -1 & 1 & 1 & -1 \\ -1 & 1 & 1 & -1 \\ 1 & -1 & -1 & 1 \end{bmatrix}$$

$A = A_1 = A_2$

$$k^e = \begin{bmatrix} EA & 0 & -EA & 0 & 0 & 0 \\ 0 & EA & 0 & 0 & 0 & -EA \\ -EA & 0 & EA + \frac{EA_3}{2\sqrt{2}} & \frac{-EA_3}{2\sqrt{2}} & \frac{-EA_3}{2\sqrt{2}} & \frac{EA'}{2\sqrt{2}} \\ 0 & 0 & \frac{-EA_3}{2\sqrt{2}} & \frac{EA_3}{2\sqrt{2}} & \frac{EA_3}{2\sqrt{2}} & \frac{-EA'}{2\sqrt{2}} \\ 0 & 0 & \frac{-EA_3}{2\sqrt{2}} & \frac{EA_3}{2\sqrt{2}} & \frac{EA_3}{2\sqrt{2}} & \frac{-EA'}{2\sqrt{2}} \\ 0 & -EA & \frac{EA'}{2\sqrt{2}} & \frac{-EA'}{2\sqrt{2}} & \frac{-EA'}{2\sqrt{2}} & EA + \frac{EA'}{2\sqrt{2}} \end{bmatrix}$$

Stiffness Matrix for  
bar model

b)

c) The matrices are not equal because they come from different models. While the Turner triangle considers shear stresses, the bar model can only withstand axial stresses.

d) When  $\nu \neq 0$ , plane stress and plane strain are different to one another.

$$k_{p, \text{stress}} = \frac{1}{2} \begin{bmatrix} -1 & 0 & -1 \\ 0 & -1 & -1 \\ 0 & 0 & 0 \\ 0 & 0 & 1 \\ 0 & 1 & 0 \end{bmatrix} \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 0 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{bmatrix} -1 & 0 & 1 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 1 \\ -1 & -1 & 0 & 1 & 1 & 0 \end{bmatrix}$$

$$k_{p, \text{strain}} = \frac{1}{2} \begin{bmatrix} -1 & 0 & -1 \\ 0 & -1 & -1 \\ 1 & 0 & 0 \\ 0 & 0 & 1 \\ 0 & 0 & 1 \\ 0 & 1 & 0 \end{bmatrix} \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & 0 \\ \nu & 1-\nu & 0 \\ 0 & 0 & \frac{1-2\nu}{2} \end{bmatrix} \begin{bmatrix} -1 & 0 & 1 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 1 \\ -1 & -1 & 0 & 1 & 1 & 0 \end{bmatrix}$$

An important difference is that one can recover the stress (in a plain strain) or the stress strain (in a plane stress) as a postprocess, while the bars do not consider effects occurring in the third direction.